



SIDDHARTH GROUP OF INSTITUTIONS :: PUTTUR
Siddharth Nagar, Narayanavanam Road – 517583

QUESTION BANK

Subject with Code: DESIGN OF ADVANCED CONCRETE STRUCTURES (19CE1017)

Course & Branch: M. Tech - Structural Engineering

Year & Sem: I M.TECH & II-Sem

Regulation: R19

UNIT-I

ESTIMATION OF CRACK WIDTH AND REDISTRIBUTION OF MOMENTS IN REINFORCED CONCRETE BEAMS

1. A simply supported rectangular beam 300 mm x 500 mm, having an effective span of 6 m, is subjected to UDL of 16 KN/m, inclusive of its self weight. The beam is reinforced with 3 bars of 20 mm diameter, at an effective cover of 50 mm. Assuming M20 concrete and Fe415 steel. Calculate the surface Crack width at the following locations. [12M]
 - At a point 'A' directly under a bar on tension face
 - At the bottom corner 'B' of the beam
 - At a point 'C' distant $2(d-x)/3$ from N.A, where crack width is likely to be maximum.
2. a) Advantage and disadvantages of moment redistribution [5M]
 - b) Explain moment curvature relation of reinforcement concrete sections [5M]
3. a) Calculation of Crack width in Beams [5M]
 - b) Factors affecting Crack width in beams [5M]
4. A beam AB of 4 m span and fixed at the ends, carries an UDL of 30 KN/m at collapse. Draw maximum bending moment diagram as per IS code recommendations for redistribution of moments. . [12M]
5. A simply supported T-beam span of 5 mts is subjected to a moment of 85 KN/m at mid span. The section of beam is as shown in figure. Calculate the crack width at corner A, directly under

- tension reinforcement B & the center tension face C. the materials are M20 grade concrete and Fe415 steel. . [12M]
6. a) Explain moment curvature relation of reinforcement concrete sections [5M]
- Factors affecting Crack width in beams [5M]
7. A beam of AB span 4 mts fixed at one end and freely supported at other end carrying a UDL of 30 KN/m at collapse. Draw maximum BM as per recommendation of code IS 456-2000 for redistribution of moment. . [12M]
8. (a) What are the major factors which influence the crack width in flexural members?
- a) A beam of width 450 mm, depth 700 mm cover of reinforcement 40 mm is reinforced with 3 rods of 40 mm diameter. Determine the crack width when the section is subjected to a BM of 490 Kn m at a point on the side of the beam 250 mm below the neutral axis.
9. (a) What are the major factors which influence crack-widths in flexural members? [5M]
- (b) Discuss the issues involved in designing for achieving control over thermal and shrinkage cracks in large R.C structures. [5M]
10. (a) What are the different options available to a designer with regard to control of cracking in flexural members? [5M]
- (b) Are the normal designing requirements of the code adequate for ensuring crack-width Control ? Give comment on this. [5M]
-

UNIT-II**DESIGN OF DEEP BEAMS AND CORBELS**

1. Design a single span deep beam to suit the following data and also draw reinforcement details in deep beam. [12M]
Effective span of the beam=6m
Overall depth of the beam=6m
Width of support =0.6m
Width of beam=0.4 m
Total load on beam including self weight= 400KN/m
Concrete= M20 Grade
Steel =Fe415 HYSD bars
 2. Design a 3span continuous deep beam carrying a characteristic load of 210KN/m inclusive of its self weight for the beam, clear span is 4.5 m width of support 250 mm. thickness of the beam 230 mm and Overall depth of 2.7 m. the materials are M20 Grade concrete and and HYSD reinforced of grade Fe415. [12M]
 3. Simply supported beam of 250 mm wide and 1500 mm overall depth & 2300 mm clear span is simply supported on 200 mm wide support on either side it carries UDL of 200KN/m inclusive of its self weight. Design the beam using M20 concrete and Fe415 Grade. [12M]
 4. Explain the procedure for continuous deep beam and draw the reinforcement details.[12M]
 5. Design a continuous deep beam having more than 3 spans and loaded a UDL of 180KN/m inclusive of self weight for the beam the clear span 5 mts.width of supports 300 mm beam thickness 250 mm. Overall thickness of beam is 3.5 mts. The material used are M20 HYSD bars of 415. [12M]
-
-

6. A simply supported deep beam 200 mm wide x 1800 mm overall depth and 2750 mm clear span is simply supported on 250 mm wide supports on either sides. It carries a characteristic UDL of 260KN/m inclusive of its self weight. Design and details the beam. The materials are M20 Grade concrete and HYSD reinforced of grade Fe415. .[12M]
 7. The reinforced concrete beam girder is continuous over spans of 8 m apart from c/c. It is 4.4 m deep and 330 mm thick and supports of column 900 mm width. If the girder supports a UDL of 210KN/m inclusive its own weight. Design Deep beam using M20 Grade concrete and Fe415 Grade steel. .[12M]
 8. Simply supported beam of 250 mm wide and 1500 mm overall depth & 2300 mm clear span is simply supported on 200 mm wide support on either side it carries UDL of 200KN/m inclusive of its self weight. Design the beam using M20 concrete and Fe415 Grade. .[12M]
 9. A reinforced concrete deep girder is continuous over span of 10 m apart from centre to centre. It is 4.6 m deep, 300 mm thick and the supports are columns 900 mm in width. If the girder supports a UDL of 180 Kn/m, design the reinforcement required if M20 concrete and Fe415 steel is used. [12M]
 10. Design a simply supported deep beam to the following data:
.[12M] Clear span = 4.20 m
Bearing at each end = 450 mm
Overall depth = 3500 mm Width
of beam = 250 mm Support
imposed load = 250 kN/m Use
M20 concrete and Fe415 steel.
-

UNIT-III

DESIGN OF RIBBED (VOIDED) SLABS

1. A simply supported one way ribbed slab of 5 m span is to be used for 3 KN/m^3 live load. Design the slab using M20 grade concrete and HYSD bars of grade Fe 415. .[12M]
 2. Design a continuous ribbed slab with 3 equal spans of 5.8 m. the ribs supports on the beam with over span is 250 mm x 600 mm. take live load on the slabs is 3 KN/m^2 use M20 Grade concrete and Fe415 steel. .[12M]
 3. Explain the Analysis and Design procedure for ribbed Slabs .[12M]
 4. A simply supported one way ribbed slab of 6 m span is to be used for 5 KN/m^3 live load. Design the slab using M20 grade concrete and HYSD bars of grade Fe 415. .[12M]
 5. Write short notes on: . [12M]
 - (a)Shear effect in two-way slab with beams.
 - (b)Flat slabs with opening.
 - (c)ACI guidelines for shear calculations.
 - (d)Strengthening of columns for shear and torsion
 6. Short note on:
 - (a)Shear due to unbalanced moments.
 - (b)Effect of opening in flat slab.
 - (c)Strengthening of column areas for moment transfer.
 - (d)Shear design guidelines as per ACI code.
 7. A simply supported one way ribbed slab of 5m span is to be used for 3 kn/m^2 live load design the slab using M20 grade concrete and Fe 415 HYSD bars. . [12M]

Ribs are spaced at 450mm c/c.
The thickness of topping as 60 mm.
Width of rib as 120mm.
Over all depth is 300mm.
 8. Design a continuous ribbed slab with 3 equal spans at 5.8m the ribs support on the beam with over span is 250mm x 600mm. take live load on the slab is 3 Kn/m^2 use M20 grade concrete and Fe415 steel. . [12M]
-

Ribs are spaced at 450mm *c/c*
The thickness of topping as 60 mm
Width of rib as 120mm
Over all depth is 300mm

9. A simply supported one way ribbed slab of 6m span is to be used for 5 kn/m^2 live load design the slab using M25 grade concrete and Fe 415 HYSD bars. .[12M]

Ribs are spaced at 450mm *c/c*.
The thickness of topping as 50 mm.
Width of rib as 120mm.
Over all depth is 250mm.

10. Design a continuous ribbed slab with 4 equal spans at 5.8m the ribs support on the beam with over span is 250mm x 600mm. take live load on the slab is 4 Kn/m^2 use M25 grade concrete and Fe415 steel. . [12M]

Ribs are spaced at 450mm *c/c*
The thickness of topping as 60 mm
Width of rib as 120mm
Over all depth is 300mm

UNIT-IV

DESIGN OF GRID FLOORS AND FLAT SLAB

1. A flat plate 7×6 m panel on 500×500 mm column has a slab thickness of 180 mm, designed for a load of 9.3 kN/m^2 . Check for the safety of slab in shear and also determine the necessary stirrups for reinforcing the slab. Adopt M25 and Fe 415. . [12M]
2. A flat plate with 7.5×7.5 m panels on 500×500 mm columns has a slab thickness of 180 mm, designed for a total load of 9.0 kN/m^2 . Check the safety of slab in shear and also find the stirrups for reinforcing in the slab. Use M25 and Fe415. . [12M]
3. R.C.Grid floor is to be designed to cover a floor area of $12 \text{ m} \times 8 \text{ m}$. The spacing of ribs in mutually perpendicular directions is 1.5 m centre to centre. Live load on the floor is 2 kN/m . Analyze the grid floor by IS 456 methods and enumerate the suitable reinforcements.[12M]
4. Design an interior panel of a flat slab of size $5 \text{ m} \times 5 \text{ m}$ without providing drop and column head. Size of column is $500 \times 500 \text{ mm}$ and live load on the panel is 4 kN/m^2 . Take floor finishing load as 1 kN/m^2 . Use M20 Concrete and Fe 415 steel. [12M]
5. A reinforced grid floor is to be design to cover of $12 \text{ m} \times 18 \text{ m}$ the spacing of the ribs is mutually perpendicular direction is 1.5 m c/c . live load of floor is 3 kN/m^2 adopt M20 grade concrete and Fe415 steel assume ends are simply supported analyze the the grid floor using IS456:2000 method and design suitable reinforcement in the grid floor. . [12M]
6. A reinforced grid floor is to be design to cover of $16 \text{ m} \times 22 \text{ m}$ the spacing of the ribs is mutually perpendicular direction is 2 m c/c . live load of floor is 3 kN/m^2 adopt M25 grade concrete and Fe415 steel assume ends are simply supported analyze the the grid floor using IS456:2000 method and design suitable reinforcement in the grid floor. . [12M]
7. A reinforced grid floor is to be design to cover of $9 \text{ m} \times 12 \text{ m}$ the spacing of the ribs is mutually perpendicular direction is 1.1 m c/c . live load of floor is 2 kN/m^2 adopt M20 grade concrete and Fe415 steel assume ends are simply supported analyze the the grid floor using IS456:2000 method and design suitable reinforcement in the grid floor. . [12M]
8. Design the interior panel of a flat-slab floor system for a ware house 24 m divided into panels of $6 \text{ m} \times 6 \text{ m}$. . [12M]
 Loading class = 5 kN/m^2
 Materials : M20 grade concrete, Fe 415 HYSD bars
 Column size = 400 mm diameter
9. Design the exterior panel of a flat-slab floor system for a ware house 24 m divided into panels of $6 \text{ m} \times 6 \text{ m}$. . [12M]

Loading class = 4Kn/m^2

Materials : M25 grade concrete, Fe 415 HYSD bars

Column size = 400 mm diameter

10..A flat plate with 7.5*7.5m panels on 500*500mm columns has a slab thickness of 180 mm, designed for a total load of 8.0 kN/m². Check the safety of slab in shear and also find the stirrups for reinforcing in the slab. Use M20 and Fe415 .[12M]

UNIT-V

DESIGN OF PLAIN CONCRETE WALLS AND SHEAR WALLS

1. A plain braced concrete wall of dimensions 8 m high, 6m long and 200 mm thick is restrained against rotation at its base and unrestrained at the ends. If it has to carry a factored total gravity load of 200 KN and a factored horizontal load of 8 KN at top. Check the safety of the wall. Assume M20 concrete and Fe 415 steel. . [12M]

2. Estimate the reinforcement of a shear wall of length 4.16 m and thickness 250 mm is subjected to the following forces $f_{ck}=25 \text{ N/mm}^2$, $f_y= 500 \text{ N/mm}^2$.[12M]

S.No	Loading	Axial Load (KN)	Shear Force (KN)	Bending Moment (KN-m)
1	DL+LL	1950	600	20
2	Seismic Load	250	4800	70

3.Design a shear wall of length 5.0 m and thickness 250 mm subjected to the forces given below and the wall is a high wall with the following loadings. Use M25 and Fe415. .[12M]

S.No	Loading	Axial Load (KN)	Shear Force (KN)	Bending Moment (KN-m)
1	DL+LL	1950	500	20
2	Seismic Load	200	4500	80

4.Design a shear wall subjected to $P_u=12000\text{kn}$ and $M_u=11000\text{kn.m}$. the materials used are M30 grade concrete and Fe 415 steel and thickness of wall is 200mm and length is 6m design the wall

- Using interaction chart . [12M]
- Using elastic stress distribution design end portion of height 600mm
- Assume end zone to resist moment and 500mm X 500mm column at end zone.

5. Design a shear wall subjected to $P_u=10000\text{kn}$ and $M_u=8000\text{kn.m}$. the materials used are M30 grade concrete and Fe 415 steel and thickness of wall is 150mm and length is 5m design the wall

- Using interaction chart
 - Using elastic stress distribution design end portion of height 600mm
 - Assume end zone to resist moment and 500mm X 500mm column at end zone.
6. Design a shear wall subjected to $P_u=15000$ kn and $M_u=11000$ kn.m. the materials used are M25 grade concrete and Fe 415 steel and thickness of wall is 200mm and length is 6m design the wall. [12M]
- Using interaction chart
 - Using elastic stress distribution design end portion of height 600mm
 - Assume end zone to resist moment and 500mm X 500mm column at end zone.
7. Briefly explain the classifications of shear wall with neat sketch? .[12M]
8. Explain the design procedure to design the shear wall? .[12M]
9. A plain braced concrete wall of dimensions 10 m high, 6m long and 200 mm thick is restrained against rotation at its base and unrestrained at the ends. If it has to carry a factored total gravity load of 250 KN and a factored horizontal load of 10 KN at top. Check the safety of the wall. Assume M25 concrete and Fe 415 steel. .[12M]
10. Design a shear wall of length 5.0 m and thickness 250 mm subjected to the forces given below and the wall is a high wall with the following loadings. Use M25 and Fe415. . [12M]

S.No	Loading	Axial Load (KN)	Shear Force (KN)	Bending Moment (KN-m)
1	DL+LL	1950	500	20
2	Seismic Load	200	4500	80